

THE INFLUENCE OF TEMPERATURE ON THE UNIAXIAL COMPRESSIVE STRENGTH OF ROCKS

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Abstract

This study focuses on determination of the temperature effect on uniaxial compressive strength of different rock types. For our laboratory tests, we chose local greywacke from the Kobeřice quarry which is situated 10 km south of Prostějov. For comparison, datasets representing other rock types have been gathered from the literature cited. As shown in this study, the rock type structure, composition or the processes that formed the rock can influence how the compressive strength changes across the temperature range. Results of similar analyses can be utilized during the design of underground constructions or for assessment of the fire damage on a rock-based constructions.

Keywords

Rock mechanics, uniaxial compressive strength, greywacke, temperature influence

1 INTRODUCTION

One of the many obstacles that have to be overcome during construction design is elevated temperature. Usually, it is associated with unintentional fires happening in the construction itself or in its vicinity. Thus, the knowledge of the heated rock behaviour can be needed to assess the fire damage. These types of constructions usually include historical buildings with stone as the main building material (e.g. the Notre Dame Cathedral fire in 2019) or some special cases of tunnels with a limited use of concrete lining [1]. In some cases of underground constructions, such as deep geological repositories of nuclear waste or enhanced geothermal systems (EGS), the elevated temperature is a necessary part of the intended use of the designed construction. Rock-based parts of both of the construction types can be exposed to temperatures reaching up to lower hundreds of degrees and therefore have to be designed and protected accordingly [2].

The objective of this small study was to conduct laboratory examinations of the uniaxial compressive strength (UCS) of greywacke and its dependence on temperature. The results were then compared with other studies performed on various rocks. An insight into rock changes during heating was also presented along with a possible explanation for the greywacke strength changes.

2 PRESENT STATE OF RESEARCH

Thermomechanical studies of rock properties have been performed since the 1970s. Emphasis has been placed on uniaxial compressive strength and elasticity modulus studies, with other rock properties studied less frequently. In most studies, the cylindrical rock samples were heated to the target temperature with gradients between 2 and 10 °C/min. After reaching the target temperature, it was held for 1 to 12 hours to ensure uniform heating throughout the body. The studies differ in terms of using different cooling methods, such as a slow air cooling or quick cooling with water simulating firefighting [3].

According to some researches, the process of heating a rock can be divided into three phases [4]. The strength of the rock in the first stage is mostly dependent on the thermal stability of the individual minerals and their expansion. In the second stage, the strength depends on the intergranular contact and the build-up of stress due to the volumetric changes inside the rock mass. The third stage is influenced by the minerals' degeneration and destruction. Based on the rock's behaviour during these stages, the rock types were divided into two groups [5]: crystalline and clastic rocks in the first group and carbonated rocks in the second group. The first group of rocks withstands the first stage of heating without any significant changes, with the second stage usually showing a slight increase in strength. During the third stage, structural defects are formed and the strength decreases until rock

destruction. The carbonate rocks start to slowly degrade at relatively low temperatures owing to their chemical composition.

As for the tested rock types, most of the studies found during our research focused on granites (e.g. [1], [2], [6], [7], [8], [9] or [10]). Several studies focused on testing limestone (e.g. [3], [11], [12] or [13]), sandstone (e.g. [8], [12], [14] or [15]) or marble (e.g. [8], [12] or [13]). The least tested rock types include diabase [1], schist [1] and mudstone [16] with only one study performed. Greywacke has never been tested according to our research.

3 METHODOLOGY

For the purpose of the study, two types of greywacke from the Kobeřice quarry (49°22'02.9"N, 17°06'19.2"E) were used. Blocks made from fine grained greywacke were sampled directly from the quarry. The bodies were machined using a diamond core drill bit with a 46 mm diameter and a circular saw with a diamond cutting disc. A second set of samples was made of four coarser greywacke samples with a diameter of 38 mm from the depository of our institute. All of the samples had a length-to-diameter ratio of 2.0. The fine greywacke bodies were divided into four groups and heated to 200, 300 and 400 °C with one set unheated. The coarser samples were heated to 400, 800 and 1200 °C respectively, with one also left unheated. A simple heating curve was utilised with a temperature gradient of 10 °C/min until the desired temperature was reached. Then it was held for 60 minutes after which the samples were air cooled slowly. Then the dimensions for cross section area calculation were measured using a calliper with multiple measurements for each dimension. The uniaxial compressive tests were carried out using a loading rate of 800 and 1500 N/s based on the body diameter.

Individual datasets were then collected from the literature cited and from our laboratory tests. As for the interpretation of the results, an unheated mean uniaxial compressive strength was determined for each dataset. All of the strength values were then divided by the mean values, split based on the rock type and plotted against the temperature. The data were then fitted using multiple order polynomials for easier interpretation.

4 RESULTS

The coarser greywacke sample set clearly illustrates the drastic structural and visual changes caused by the high temperatures (see Fig. 1). Heating to 800 °C causes only slight changes in the rock structure, with slight sample volume increase. The main rock degeneration happens in temperatures of around 1200 °C when the rock starts to melt, the body centre becomes porous and causes significant volume increase with surface tensile cracks.

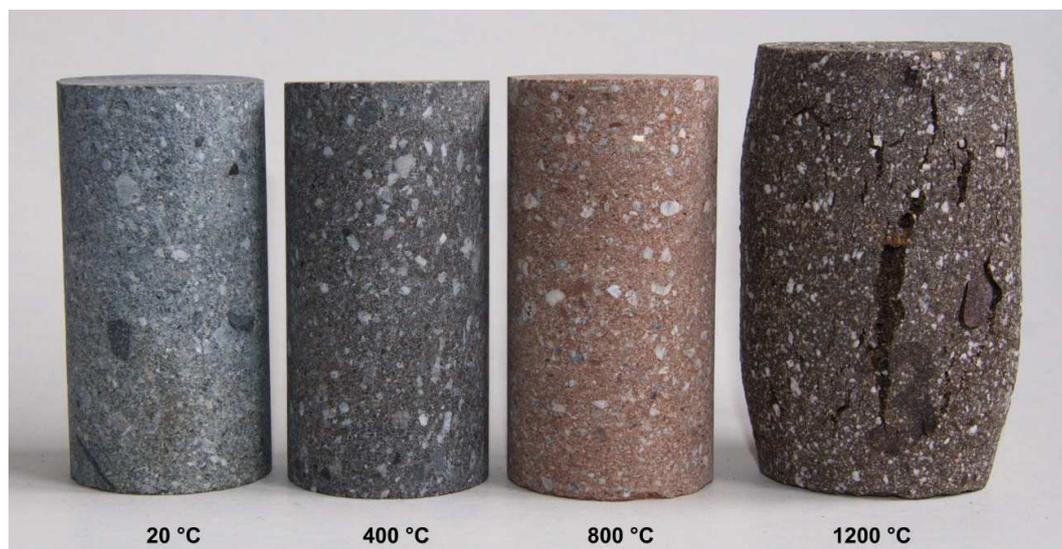


Fig. 1 The coarser greywacke samples after heating.

The results of the greywacke laboratory tests are summarized in Tab. 1. An outlying strength value had to be excluded from the fine greywacke heated to 200 °C dataset. The unheated sample strength value for coarser greywacke also had to be excluded because of a hidden defect in the sample. Instead, a strength value from previous research performed on the same sampled set of blocks was used as a substitute [17].

Tab. 1 The greywacke uniaxial compressive strength values in MPa, the substituted value is marked with *.

Set	Unheated (20 °C)	200 °C	300 °C	400 °C	800 °C	1200 °C
Fine	142.1	124.8	127.3	155.5		
	150.9	129.2	137.1	125.1		
	152.7	119.6	151.3	137.1		
	155.1	104.6	139.7	140.4		
	132.7	-	107.2	137.6		
Coarser	191.0*			197.5	165.3	17.0

The greywacke compressive strength values along with the other rock types from cited literature were converted to ratios of heated to unheated values. Plots of the ratio against the temperature were then presented with polynomial fits signifying the UCS trend (see Fig. 2). No polynomial was used for rock types with only one dataset (n = 1), the values were connected with a line instead.

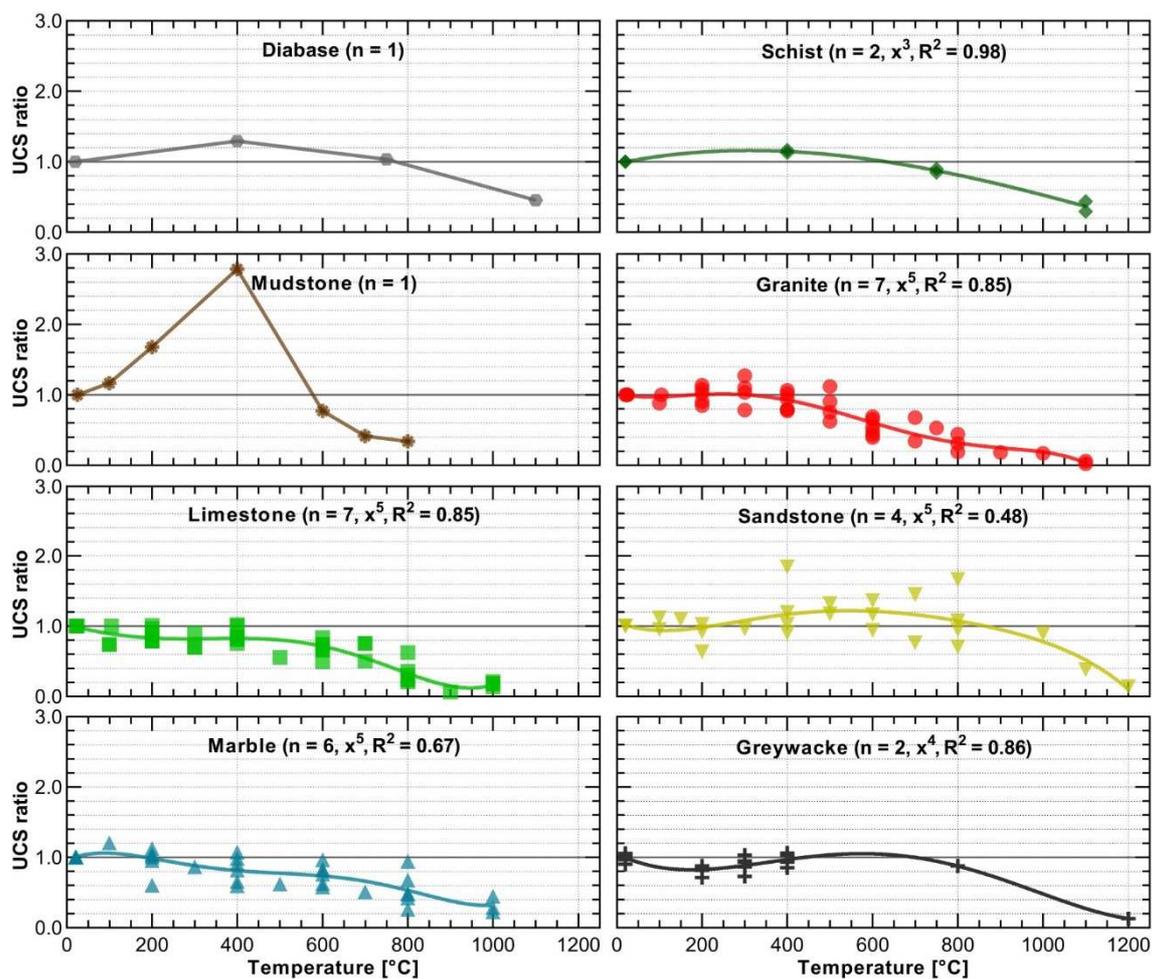


Fig. 2 Graphs of uniaxial compressive strength (UCS) ratio dependence on temperature. Number of individual datasets for each rock type is represented by n. Degree of the used polynomial is marked by x^{degree}. Coefficient of determination is also shown.

5 DISCUSSION

There are significant differences in individual rock types' behaviour when heated. Diabase and schist show an increase in strength at 400 °C followed by a slow degradation. Mudstone follows a similar trend with the increase being more significant to almost three times the unheated strength value. Granite holds relatively well

to temperatures below 400 °C with an almost constant decrease afterwards, caused mostly by an α - β quartz phase transition happening at 573 °C and a formation of microcracks [7]. Limestone and marble start to slowly decompose even in lower temperatures, as stated before. Marble seems to resist decomposition until around 200 °C which could be caused by its metamorphic state. Sandstone holds its strength until 300 °C with a significant increase in strength to 600 °C. Around 800 °C the strength falls back to the unheated value and decreases afterwards. The strengthening of sandstone could be caused by fusion of the individual sand grains as shown in [8].

Greywacke seems to differ from the other presented rock types. The strength shows a slight decrease in the range around 200 to 300 °C. In the range between 400 and 700 °C, the strength again equals the unheated value and afterwards greywacke steadily weakens. Greywacke differs from sandstone with a slight state of metamorphosis. There have been several different geological studies of local greywackes (Vitrinite reflectance [18], Illite crystallinity index [19] and presence of an authigenic monazite [20]) determining the individual formations temperature history. According to them, the Myslejovice formation where we sampled the blocks should have been heated to maximum temperature of 200 to 300 °C in the past. This corresponds to the compressive strength decrease we observed and possibly explains it. Greywacke could have a thermal memory which causes it to be weaker against temperatures it already underwent. The increase in strength after the 400 °C is not as pronounced as in sandstone. However, our dataset does not cover that temperature range in detail. As mentioned before, greywacke heated up to 1200 °C is already significantly decomposed. Development of many pores in the sample core can be observed, resulting in a volume increase and surface cracking.

6 CONCLUSION

The greywacke compressive strength seems to develop differently from the other rock types presented in this paper. In general, greywacke holds relatively well to the temperatures up to almost 800 °C. Unexpectedly, its strength drops in the range of around 200 to 300 °C. This phenomenon is possibly related to the maximum paleotemperature it underwent during its formation.

As for possible future research, higher temperature ranges between 500 to 800 °C and even 800 to 1200 °C could be studied in more detail. According to our data, greywacke does not show a significant increase in strength in higher temperatures as sandstone. However, this could be due to the fact that only a few samples were heated to these temperatures. Another possibility for further research is to determine the effect of different means of cooling on the strength of greywacke.

Acknowledgement

This paper was written with the support of Brno University of Technology – Faculty of Civil Engineering, within grant No. FAST-S-23-8272.

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